LAKE TECHNICAL COLLEGE BLDG 2

EVERGREEN CM

FO# 20468

Building 2 of 4

Notes:

1. All interior structural steel frames must align with each other at all interior spaces that receive interior metal liner panels.
2. All exterior structural steel frames must have the exterior metal faces in alignment with each other.
3. All interior faces of structural frames:
   - On North and South walls must align with each other on interior of building.
   - All interior faces of structural frames going East and West must align with each other.
4. Please furnish matching base plate revisions to allow General Contractor to form anc pour concrete foundations to accommodate the above revisions, i.e.
   - North and South walls, 8" x 1'-1-1/2"
   - East and West walls, 8" x 8-1/8"
5. Please clarify dimension between column lines 5.1 and 6 on Anchor Bolt Plan.
6. Does wall juncture at column line 4.9 and G. require a structural column?

DICKERSON ARCHITECTS, INC.
☐ APPROVED ☐ CORRECT & RESUBMIT
☐ APPROVED AS NOTED ☐ REJECTED - RESUBMIT
SUBMITTALS ARE REVIEWED FOR COMPLIANCE WITH DESIGN CONCEPT IN THE CONTRACT DOCUMENTS. G. C. MUST COMPLY WITH CONTRACT DOCUMENTS.
BY: JSD DATE: 05/11/17

Tyler B. Theuret, P.E. Digitally signed by Tyler B. Theuret, P.E. Date: 2017.04.20 15:38:35 -04'00'

TYLER B. THEURET LICENSE No. 81869 STATE OF FLORIDA PROFESSIONAL ENGINEER
4/20/17

CORLE BUILDING SYSTEMS
404 Sarah Furnace Road - Imler, PA 16655 (814) 276 - 9611

INDEX OF DRAWINGS

Page	Drawing Title	REV NO.
	Cover Page	01
1	Specifications	01
2	Anchor Bolt Plan & Layout	02
3	Rigid Frame Reactions	01
4	EndWall Reactions, Design Criteria	01
5	Anchor Bolt Details	02
6	Roof Framing	01
7	Roof Panel Layout	01
8	Rigid Frame #1	01
9	Rigid Frame #2	01
10	Rigid Frame #3	01

Page	Drawing Title	REV NO.
11	Rigid Frame #4	01
12	Rigid Frame #5	01
13	Front Sidewall Framing	01
14	Back Sidewall Framing	01
15	Left Endwall Framing	01
16	Right Endwall Framing	01
17	Liner Panel Layout	01
18	Roof Liner Panel	01
19	Detail Page #1	01
20	Detail Page #2	01
21	Detail Page #3	01

Page	Drawing Title	REV NO.
22	Detail Page #4	01
23		0
24		0
25		0
26		0
27		0
28		0
29		0
30		0
31		0
32		0

GENERAL

All materials included in the Metal Building System are in accordance with the manufacturer's standard materials and details unless otherwise specified on the order documents. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 2.1)

DESIGN RESPONSIBILITY

The manufacturer is responsible only for the structural design of the Metal Building System it sells to the purchaser / customer. Neither the manufacturer nor the manufacturer's engineer is the design professional or engineer of record for the construction project. The manufacturer is not responsible for the design of any component or materials not sold by it, or their interface and connection with Metal Building System unless such design responsibility is specifically required by the order documents. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.1)

FOUNDATION DESIGN AND ANCHOR BOLTS

The manufacturer is not responsible for the design, materials, and workmanship of the foundation. The anchor bolt plans prepared by the manufacturer are intended to show only the anchor bolt location, diameter (based on ASTM A36 bolts), and quantity required to connect the Metal Building System to the foundation. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.2)

It is the responsibility of the end customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and / or associated items embedded in the concrete foundation, as well as foundation design based on the loads imposed by the Metal Building System, or other imposed loads, and the bearing capacity of the soil and other conditions of the building site (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.2)

U.S. -Anchor bolts shall be accurately set to a tolerance of +/- 1/8 in both elevation and location (AISC Code of Standard Practice for Steel Buildings and Bridges).

Canada -Anchor bolts shall be accurately set in accordance with CISC Code of Standard Practice, June 2008, Clause 7.7.1

ADJACENT EXISTING BUILDINGS

The manufacturer does not investigate the influence of the Metal Building System on adjacent existing buildings or structures. The end customer assures that such buildings and structures are adequate to resist snow loads or other conditions as a result of the presence of the Metal Building System. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.5)

SHOP-PRIMED STEEL

All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by corrosion resistant coating are painted with one coat of shop primer. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum the hand tool cleaning method SSPC-SP2 (Steel Manual Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop-primed steel should be placed on blocking to prevent contact with the ground, and so positioned as to minimize water holding pockets, dust, mud an other contamination of the primer film. Repairs of damage to primed surfaces and or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. (CISC Code of Standard Practice, June 2008, Clause 6.8; (MBMA 2012 Metal Building Systems Manual, Part IV, Section 4.2.4).

ERECTION-GENERAL

The erector, by entering into contract to erect the building, holds itself out as skilled in the erection of Metal Building Systems and is responsible for complying with all applicable local, federal, and state construction and safety regulations including OSHA regulations as well as any applicable requirements of local, national, or international union rules or practices. (CISC Code of Standard Practice, June 2008, Clause 7.2; (MBMA 2012 Metal Building System Manual, Part IV, Section 6.9).

The erector shall erect the Metal Building System in accordance with the erection drawings, the Erection and Detail Manual (February 2012), and / or the *Seam-Lok* Technical – Erection manual (May 2012) as furnished by the manufacturer. The aforementioned erection information is intended to illustrate the layout of the framing members, provide the associated connection details, and suggests sequence of erection. It is not intended to specify any particular method of erection to be followed by the erector. The erector remains solely responsible for the safety and appropriateness of all techniques and methods utilized by its crews in the erection of the Metal Building System. The erector is responsible for supplying any safety devices such as scaffolds, runways, nets, et, which may be required to safely erect the Metal Building System. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.9) The manufacturer expressly disclaims any responsibility for injury to persons in the course of erection or for damages to the product itself. Field erection of a Pre-Engineered Metal Building, as in all construction projects, involves hazards to persons within the area of the construction and risk of damage to the property itself. Only experienced persons who are skilled and qualified in the erection of Metal Building Systems should be permitted to field-erect a building due to the hazards of this construction activity. The manufacturer is not responsible for the erection of the Metal Building System, the supply of any tools or equipment, or any other field work. The manufacturer provides no field supervision for the erection of the structure nor does the manufacturer perform any intermediate or final inspections of the Metal Building System during or after erection.

The erector shall furnish temporary guys and bracing where needed for squaring, plumbing, and securing the structural framing against loads, such as wind loads acting on the exposed framing as well as loads due to erection equipment and erection operation, but not including loads resulting from the performance of work by others. Bracing furnished by the manufacturer for the Metal Building System cannot be assumed to be adequate during erection. Temporary supports such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, erected, and installed by the erector. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 7.10.3; CISC Code of Standard Practices, June, 2008, Clause 1.5; MBMA 2012 Metal Buildings System Manual, Part IV, Section 6.2.1.5).

ERECTION TOLERANCES

U.S. ; Erection tolerances are those set forth in AISC code of standard practice except individual members are considered, plumb, level and aligned if the deviation does not exceed 1:500. (AISC Code of Standard Practice for Steel Buildings and Bridges April 14, 2010 Section 7.13.1; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.8)

Canada; Erection tolerances are those set forth in CISC Code of Standard Practice except individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. (CISC Handbook of Steel Construction, Tenth Edition, Second Revised Printing, Part 1, Clause 29.3; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.3)

BOLT TIGHTENING

The proper tightening and inspection of all fasteners is the responsibility of the erector (Reference RCSC for structural joints using high strength bolts; August 1, 2014). All high strength (ASTM A325, ASTM A490) bolts and nuts must be tightened by the "turn-of-the-nut" method unless otherwise specified by the end customer in the contract documents. Inspection of high strength bolt and nut installation by other than the erector must also be specified in the contract documents and the erector is responsible for ensuring that the installation procedures are compatible prior to the start of erection (CISC Handbook of Steel Construction, Tenth Edition, Second Revised Printing, Part 1, Clause 23.8.2), (MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.9).

MATERIALS	ASTM DESIGNATION	MINIMUM YIELD	MATERIALS	ASTM DESIGNATION	MINIMUM YIELD
Hot-Rolled Mill Sections	A 36, A 572, A 992	Fy = 36 ksi and/or 50 ksi	Roof and Wall Sheeting	A 792, Gr. 50 Class 1 A 792, Gr. 80	Fy = 50 ksi Fy = 80 ksi
Structural Steel Plates	A 572, A 1011	Fy = 55 ksi	Mild Steel Bolts	A 307	Fy = 36 ksi
Structural Steel Bars	A 572 or A 529	Fy = 55 ksi	High Strength Bolts	A 325-N A 490-N	Fy = 92 or 81 ksi N/A
Cold Formed Light Gauge Shapes	A 653 Gr. 55	Fy = 55 ksi	Anchor Rods (If supplied)	A 36	Fy = 36 ksi
Cable Bracing	A 475, EHS	N/A	Pipe and Hollow Structural Sections	A 500 Gr. B	Fy = 42 ksi, 46 ksi
Rod Bracing	A 36	Fy = 36 ksi			

CORRECTION OF ERRORS AND REPAIRS

The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping, and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 7.14; CISC Code of Standard Practice, June 2008, Clause 7.15; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.10).

DRAWING DISCREPANCIES

In case of discrepancies between the manufacturers steel plans and plans for other trades, the manufacturers steel plans govern. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 3.3; CISC Code of Standard Practice, June 2008, Clause 3.4; MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.1).

DELIVERIES

Delivery of any material by the manufacturers carrier, a common carrier, or to purchasers/ customers own leased, chartered, or authorized conveyance shall constitute delivery to builder, and thereafter, such material shall be at builders risk. If builder chooses to use its own, or private carrier, it shall be solely responsible for compliance with all applicable government regulations. All charges shall be borne by the builder. The manufacturers responsibility for damage or loss ceases upon delivery of shipment to carrier. The manufacturer will endeavor to deliver on the required date. The manufacturers truck is not considered as being late if deliveries are between 8am – 12pm (morning) and 12pm – 5pm (afternoon). However, the manufacturer cannot be held responsible for circumstances beyond our control. For deliveries via the manufacturers truck, the manufacturer will only honor claims that were approved by the customer service department at the time of delivery. For deliveries via contract carriers, it is the responsibility of the customer to file claims with the carrier. The manufacturer cannot assume any liability for the claim.

SHORTAGES

The purchaser /customer should make an inspection upon arrival of all building components. The purchaser/customer must note on the freight bill any missing item(s) and notify the manufacturers customer service department immediately; otherwise, the manufacturer cannot be held responsible for any shortages. If any item is damaged, note on the bill of lading and file a claim with the freight agent. Concealed shortages must be reported to the manufacturers customer service department within the following time frames (date from receipt of first delivery), based on the project shipment size, i.e., number of truck loads used in delivery.

1 to 3 loads...2 weeks 4 loads and over...3 weeks The manufacturers responsibility for shortages expires at the end of these time periods.

FABRICATION ERRORS

The purchaser/customer is responsible for contacting the customer service department to advise the manufacturer of fabrication problems and corresponding cost estimates. The manufacturer will be responsible for providing the builder with verbal approval to proceed with appropriate field corrections. This will be done in a timely manner. IF THE BUILDER PROCEEDS WITH CORRECTIVE WORK WITHOUT THE MANUFACTURERS APPROVAL, HE DOES SO AT HIS OWN RISK. The manufacturer shall not be responsible for any claims where the purchaser/customer has not documented the problem, its correction, and reasonable costs for repair, and submitted this documentation for payment within 30 days of the occurrence.

INVOICE PAYMENT

By acceptance of the materials of services set forth in the invoice, the purchaser/customer agrees to pay the invoice amount within the time period specified on the invoice. AT NO TIME IS IT ACCEPTABLE TO DEDUCT A BACK CHARGE OR SHORTAGE FROM AN INVOICE.

SAFETY PROCEDURES

The manufacturer is committed to manufacturing a quality product that can be erected safely. Although good job site practices and a commitment to safety by the erector are beyond the control of the manufacturer, the manufacturer highly recommends the erector provide good, safe working conditions on the job site. The erector should follow all local, state, and federal health and safety regulations at all times. Accident prevention practices should be implemented and each employee should know emergency procedures. The manufacturer also recommends daily meetings to discuss erection safety procedures. For additional information concerning federal health and safety regulations, contact the occupational safety and health administration (osha).

U.S. Department of Labor
Occupational Safety and Health Administration
200 Constitution Avenue, N.W.
Washington, DC 20210
www.osha.gov

The manufacturer shall not be responsible for personal injury or property damage as a result of failure to follow all applicable safety regulations and material handling and installation recommendations.

CORE BUILDING SYSTEMS
404 Sarah Furnace Road - Imler, PA 16655 (814) 276 - 9611

LAKE TECHNICAL COLLEGE BLDG 2
12'-0" x 63'-0 1/2" x 19'-8 1/2" x 22'-8 1/2"
DATE: 3/ 7/17 | REVISION: 01
ENG: TBT | DWN: BJC | APPD: TBT

F.O.20468

LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
1	SEE CO-01	4/20/17

DRAWING STATUS

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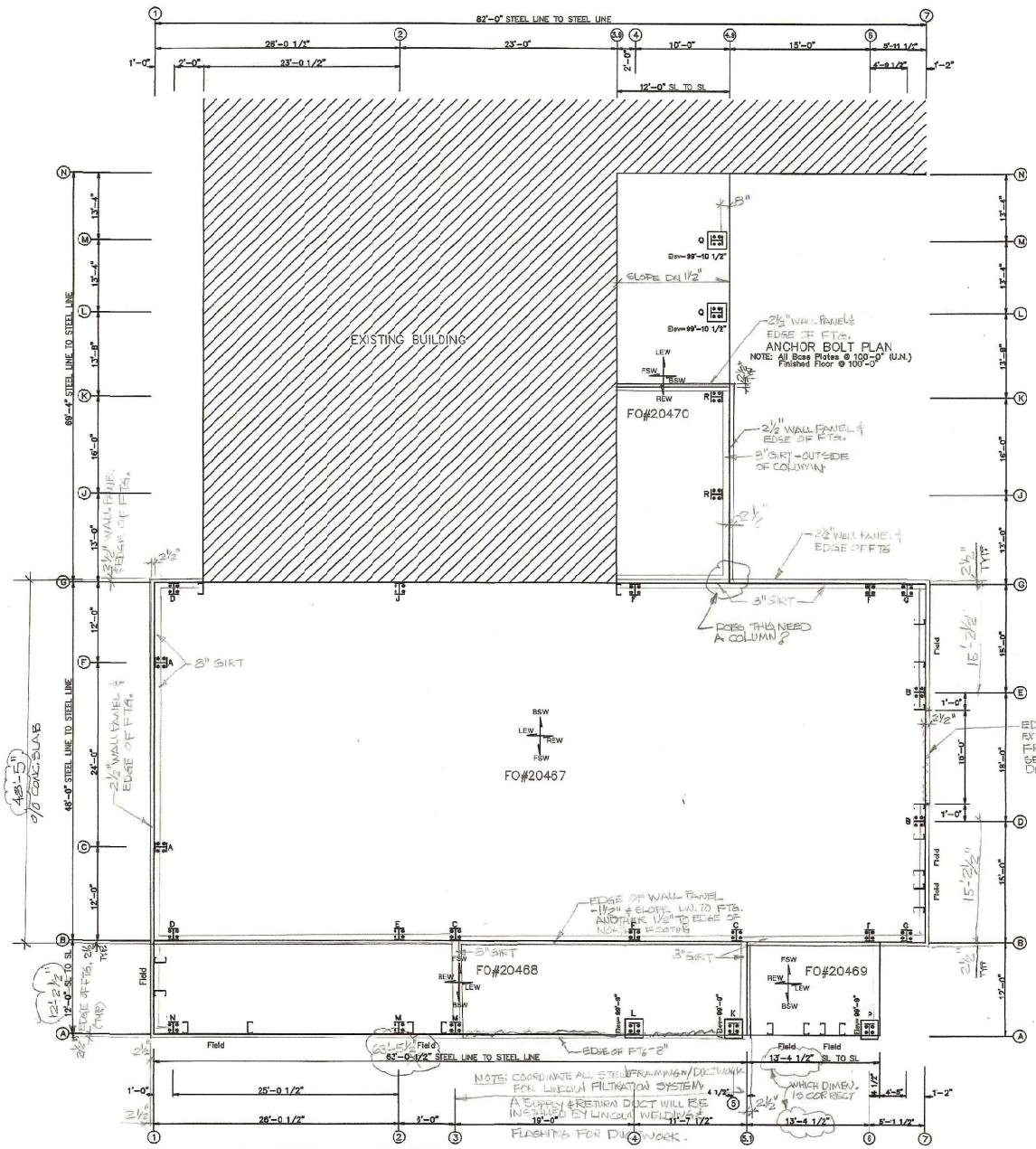
TYLER B. THEURET
LICENSE
No. 81869
STATE OF FLORIDA
PROFESSIONAL ENGINEER
4/20/17



DICKERSON ARCHITECTS, INC.
☐ APPROVED ☐ CORRECT & RESUBMIT
☒ APPROVED AS NOTED ☐ REJECTED - RESUBMIT
SUBMITTALS ARE REVIEWED FOR COMPLIANCE WITH DESIGN CONCEPT IN THE CONTRACT DOCUMENTS. O.C. MUST COMPLY WITH CONTRACT DOCUMENTS.
BY: S.D. DATE: 05/11/17

GENERAL INFORMATION

DWG. 1 OF 4
- CONTAINS SHEET 1 OF 4
- EDGE PLATES - A, B, C, D, E, F, G & J
  - A IS 6"x 8 3/8
  - B IS 8"x 7 7/8"
  - C IS 6"x 9 3/4"
  - D IS 8"x 11 3/4"
  - E IS 8"x 11 1/2"
  - F IS 8"x 1'-1 1/2"
  - G IS 8"x 1'-0 1/4"
  - J IS 8"x 11 3/4"

DWG. 2 OF 4
- NO CANOPY ON THIS ELEV. SEE 2 OF 4
- CAP PLATES - K, L, M & N
  - K IS 8"x 1'-0 1/4"
  - L IS 8"x 1'-0 1/4"
  - M IS 8"x 1'-0"
  - N IS 8"x 1'-0"

DWG. 3 OF 4
- BASE PLATE - P = 8"x 12"

DWG. 4 OF 4
- RAMP PLATE - Q & N = 8"x 12"

FLOOR PLAN - REVISED
- 63'-5 1/2" x 48'-5"

CORE ENGINEERING SYSTEMS
404 Sarah Furnace Road - Imler, PA 16655 (814) 276 - 9611

LAKE TECHNICAL COLLEGE BLDG 1
48'-0" x 82'-0" x 22'-8 1/2"
DATE: 4/12/17 | REVISION: 02
ENG: TBT | DWN: BJC | APPD: TBT

F.O. 20467

LAKE TECHNICAL COLLEGE BLDG 1

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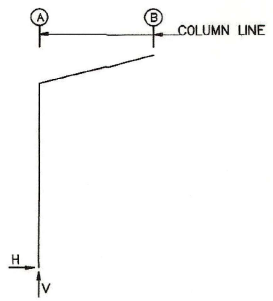
FOR CONSTRUCTION:
☐ FINAL DRAWINGS.

REVISION HISTORY

REV.	DESCRIPTION	DATE
01	SEE CO-01	4-13-17
02	UPDATED OFFSETS	4-19-17

TYLER B. THEURET
LICENSE
No. 81869
STATE OF FLORIDA
PROFESSIONAL ENGINEER
4/20/17

FRAME LINES: 5.1 4 3 2 1



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt: Dia	Base Plate (in) Width	Length	Thick	Grout (in)
5.1	A	4	0.750	8.000	12.25	0.500	-3.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt: Dia	Base Plate (in) Width	Length	Thick	Grout (in)
4	A	4	0.750	8.000	12.25	0.500	-3.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
3	A	4	0.750	8.000	12.00	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
2	A	4	0.750	8.000	12.00	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	A	4	0.750	8.000	12.00	0.500	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
5.1	A	0.0	1.2	0.0	0.4	-0.1	2.6	-0.4	-7.6	5.1	-6.2	-5.6	-2.5
4	A	0.0	1.3	0.0	0.5	-0.1	2.8	0.5	-7.0	4.8	-5.7	-5.2	-1.4
3	A	0.0	0.9	0.0	0.4	-0.1	2.3	0.4	-5.9	4.0	-4.8	-4.3	-1.1
2	A	0.0	1.1	0.0	0.5	-0.1	2.9	0.5	-7.4	5.0	-5.9	-5.3	-1.5
1	A	0.0	1.0	0.0	0.4	-0.1	2.6	0.0	-7.1	4.7	-5.7	-5.1	-2.0

Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert
5.1	A	-0.1	-1.0	4.7	-5.5	4.7	-5.5
4	A	-0.9	0.0	5.2	-5.9	5.2	-6.2
3	A	-0.7	0.0	4.3	-4.9	4.3	-5.2
2	A	-0.9	-0.1	5.3	-6.2	5.3	-6.2
1	A	-0.4	-0.6	4.6	-5.5	4.6	-5.5

CORE BUILDING SYSTEMS
404 Sarah Furnace Road - Imler, PA 16655 (814) 276 - 9611

LAKE TECHNICAL COLLEGE BLDG 2
12'-0" x 63'-0 1/2" x 19'-8 1/2" x 22'-8
DATE: 4/12/17 | REVISION: 01
ENG: TBT | DWN: BJC | APPD: TBT

F.O. 20468

LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
01	SEE CO-01	4-18-17

DRAWING STATUS

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LICENSE
No. 81869
STATE OF FLORIDA
PROFESSIONAL ENGINEER
4/20/17

ANCHOR BOLT SUMMARY

Qty	Loccte	Dia (in)	Type
O 20	Frame	3/4"	

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	Reactions in plane of wall ± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Seis	Note
L_EW	5.1								(h)
F_SW	B								(f)
R_EW	1								(h)
B_SW	A	Torsional Bracing Used							

(f)Bracing loads are applied to adjacent building
(h)Rigid frame at endwall

DESIGN INFORMATION

1. All loading conditions are examined and only the maximum / minimum H or V and the corresponding H or V are reported.
2. Positive reactions are shown in the sketch. Foundation loads are in opposite directons.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

DESIGN CRITERIA

Width (ft)	= 12
Length (ft)	= 63.04
Eave Height (ft)	= 19.71
Roof Slope (rise/12)	= 3.0:12
Building Code	= IBC 12
Local Code (State/Prov)	= FLBC 14
Dead Load (psf)	= 4.650
Collateral Load (psf)	= 5.00
Roof Live Load (psf)	= 20.00
Frame Live Load (psf)	= 20.00
Snow:	
Ground Snow Load (psf)	= 0.00
Snow Importance	= 1.00
Thermal Coefficient	= 1.20
Snow Exposure Factor	= 1.00
Slippery Roof	= N
Roof Snow Load (psf)	= 0
Wind:	
Basic Wind Speed (mph)	= 135 mph
Occupancy Category	= II - Normal
Importance - Wind	= 1.00
Wind Exposure	= C
Enclosure Classification	= P
--Internal Pressure Coefficients--	
Pressure	= 0.55
Suction	= -0.55
-----Components & Cladding-----	
Design Pressure:	
Pressure (psf)	= 55.92
Suction (psf)	= -70.24

Equivalent Lateral Brace Force Procedure.

Steel systems not specifically detailed for seismic resistance.

SEISMIC CRITERIA

Seismic Importance	= 1.00
Occupancy Category	= II - Norma
Mapped Spectral Response Accelerations	
Ss	= 0.1000
S1	= 0.0400
---Spectral Response Coefficients---	
Sds	= 0.1067
Sd1	= 0.0640
Site Class	= D
Seismic Design Category	= A
------Base Shear------	
Expanded Formula = 0.01*W	
Longitudinal Base Shear	= 0.00
Transverse Base Shear	= 0.18
--Seismic Response Coefficients--	
Frame	= 0.01
FSW	= 0.01
BSW	= 0.00
--Response Modification Factors--	
Frame	= 0
FSW	= 0
BSW	= 0

DEFLECTION LIMITS

ENDWALL COLUMN	L/ 120
ENDWALL RAFTER (Live)	L/ 180
ENDWALL RAFTER (Wind)	L/ 180
WALL GIRTS	L/ 90
PURLIN (LIVE)	L/ 150
PURLIN (WIND)	L/ 150
WALL PANEL	L/ 90
ROOF PANEL (Live)	L/ 120
ROOF PANEL (Wind)	L/ 120
Main Frame (Horiz)	L/ 60
Main Frame (Vert)	L/ 180
WIND BRACING	L/ 60
Main Frame (Crane)	L/ 100
Main Frame (Seismic)	L/ 50
SESMIC BRACING	L/ 50
PARTITION COLUMN	L/ 120
PARTITION GIRT	L/ 120
PARTITION PANEL	L/ 120

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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
01	SEE CO-01	4-19-17

DRAWING STATUS

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☐ FOR CONSTRUCTION: FINAL DRAWINGS.

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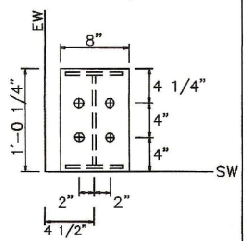
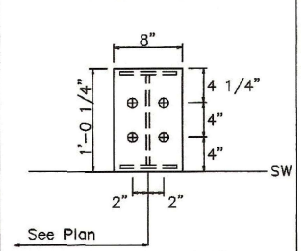
STATE OF FLORIDA
PROFESSIONAL ENGINEER
4/20/17

Anchor Bolt Dia= 3/4"



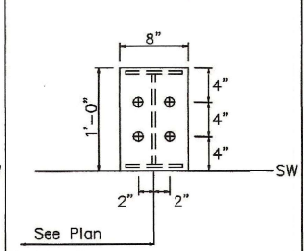
DETAIL K Base EL. 99'-9"

Anchor Bolt Dia= 3/4"



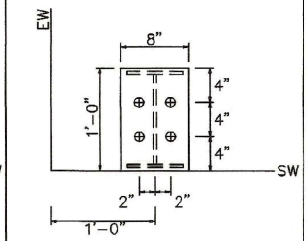
DETAIL L Base EL. 99'-9"

Anchor Bolt Dia= 3/4"



DETAIL M

Anchor Bolt Dia= 3/4"



DETAIL N

ADDITIONAL LOADING INFORMATION

Mezzanine Loads:

Dead Load	____	PSF
Collateral Load	____	PSF
Live Load	____	PSF

Crane Information:

Crane Type _____

CMAA Service Class _____

Crane capacity = _____ Kips

Bridge Weight = _____ Kips

Hoist/Trolley Weight = _____ Kips

Wheel Spacing = _____ Ft.

Additional Loads:

1. _____
2. _____
3. _____

CORE ENGINEERING SYSTEMS
404 Sarah Furnace Road - Imler, PA 16655 (814) 276 - 9611

LAKE TECHNICAL COLLEGE BLDG 2
12'-0" x 63'-0 1/2" x 19'-8 1/2" x 22'-8

DATE: 4/12/17 | REVISION: 02

ENG: TBT | DWN: BJC | APPD: TBT

F.O. 20468

LAKE TECHNICAL COLLEGE BLDG 2

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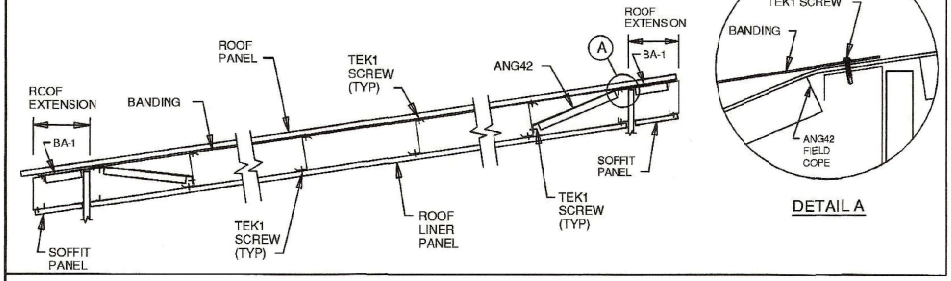
REVISION HISTORY

REV:	DESCRIPTION	DATE
01	SEE DO-01	4-13-17
02	UPDATED OFFSETS	4-18-17

TYLER B. THEURET
LICENSE
No. 81869
STATE OF FLORIDA
PROFESSIONAL ENGINEER
4/20/17

STANDARD PURLIN BRACING DETAIL
FOR STANDING SEAM ROOFS WITH ROOF LINER PANEL

NOTE 1: SPACE BANDING EVENLY ACROSS BAYS AS SHOWN



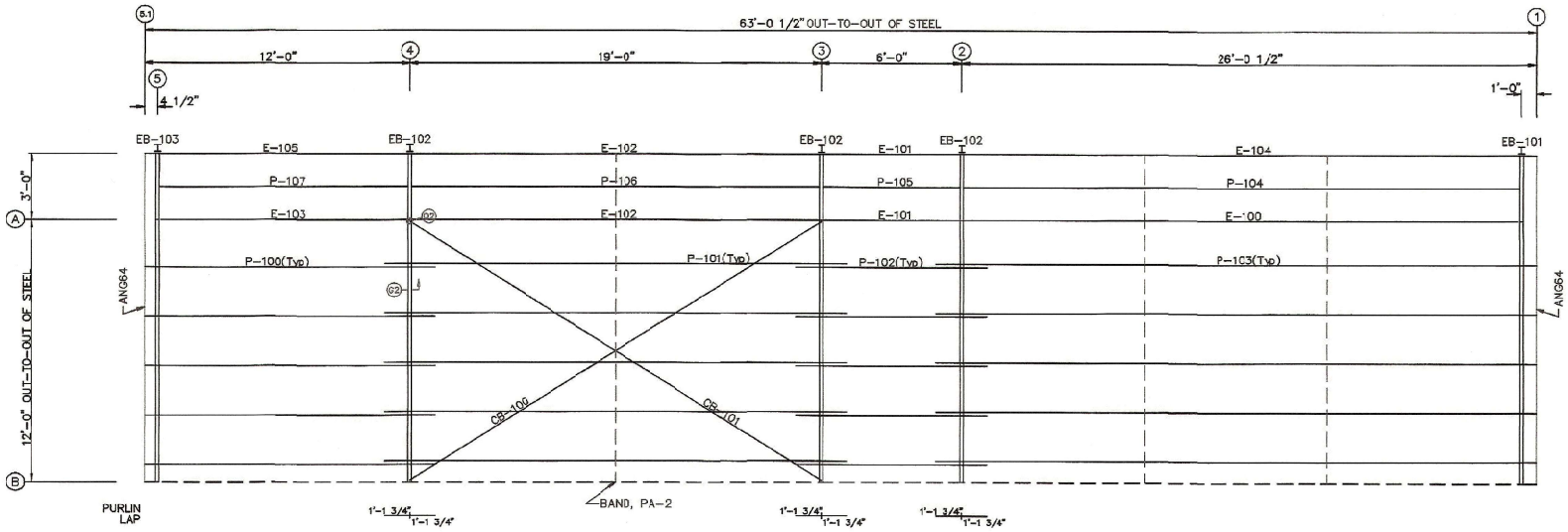
DETAIL A

EXTENSION/CANOPY BOLTS
ROOF PLAN

MARK	QUAN	TYPE	DIA	LENGTH
EB–102	4	A325	1/2"	1 1/4"
EB–103	4	A325	1/2"	1 1/4"

MEMBER TABLE
ROOF PLAN

QUAN	MARK	PART	LENGTH
1	EB–101	W8X10	4'–9 1/8"
3	EB–102	W8X10	4'–9 1/8"
1	EB–103	W8X10	4'–9 1/8"
5	P–100	8X25Z16	13'–1 1/2"
5	P–101	8X25Z16	20'–11 1/2"
5	P–102	8X25Z13	8'–7 1/2"
5	P–103	8X25Z13	27'–2"
1	P–104	8X25Z12	24'–4"
1	P–105	8X25Z16	5'–9 1/2"
1	P–106	8X25Z16	18'–1 1/2"
1	P–107	8X25Z16	10'–3 1/2"
1	E–100	8X35E10	24'–4"
2	E–101	8X35E10	5'–9 1/2"
2	E–102	8X35E10	18'–1 1/2"
1	E–103	8X35E10	10'–3 1/2"
1	E–104	8X35E10	25'–9"
1	E–105	8X35E10	11'–8 1/2"
1	CB–100	CABLE250	20'–1 1/2"
1	CB–101	CABLE250	20'–1 7/16"



ROOF FRAMING PLAN

GENERAL NOTES:
1. Screw Down Roof: Use TEK5WW screws in place of SD150 panel screws at all 10 gage purlins, eave struts, or roof joists.
2. Standing Seam Roof: Use FST#6 in place of FST#1 clip to purlin screws at all 10 gage purlins, eave struts, or at roof joists.

CORE BUILDING SYSTEMS
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LAKE TECHNICAL COLLEGE BLDG 2
12'-0" x 63'-0 1/2" x 19'-8 1/2" x 22'-8
DATE: 3/ 7/17 | REVISION: 01
ENG: TBT | DWN: BJC | APPD: TBT

F.O. 20468

LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
1	SEE CO-01	4/20/17

DRAWING STATUS

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FOR CONSTRUCTION:
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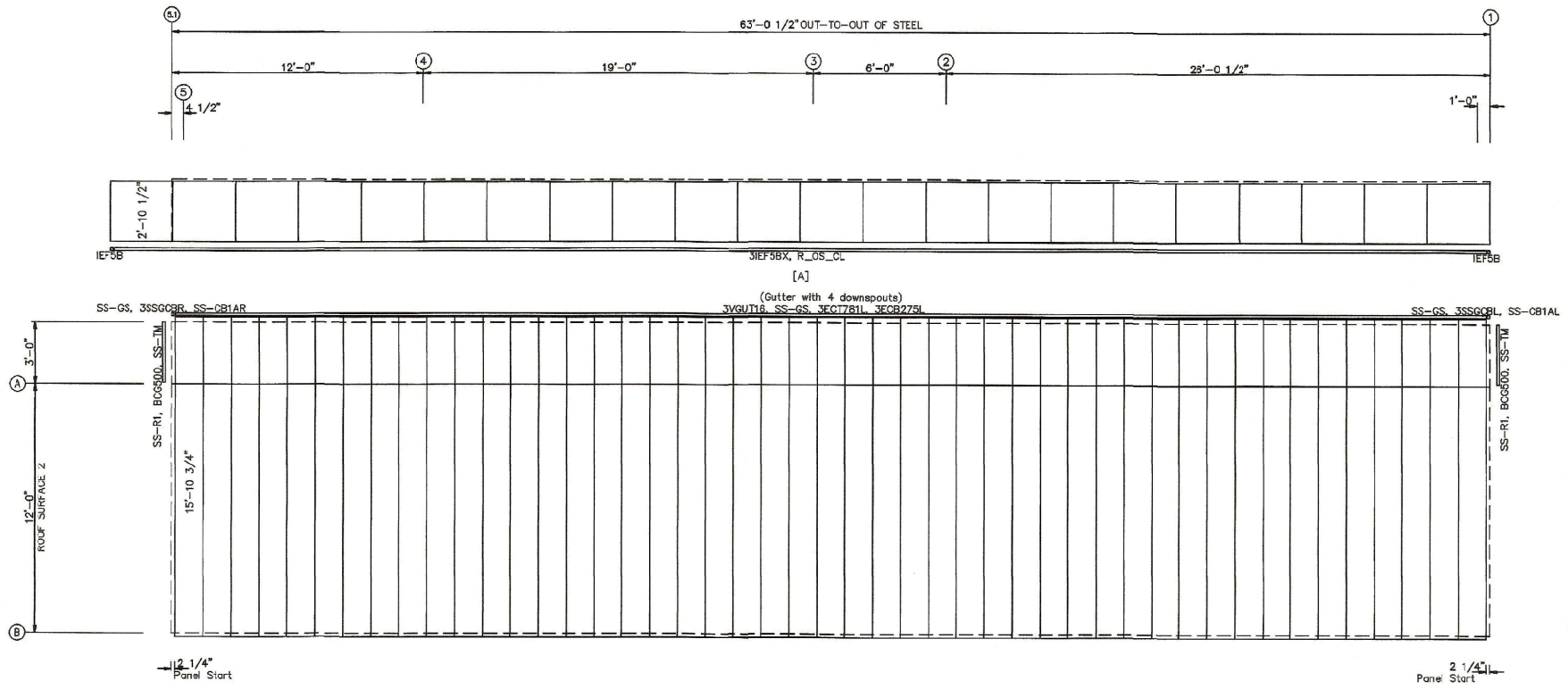
4/20/17

NOTE(S):
1.) USE (1) S-5! CLAMP AT EVERY ROOF CLIP.



ROOF SHEETING PLAN

PANELS: 22 Ga. V6 – TBD
[A] SOFFIT PANELS: 26 Ga. R – TBD

GENERAL NOTES:

Pane "Start" and "End" dimensions must be followed for the proper instalation of the gable trim(s) provided.

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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

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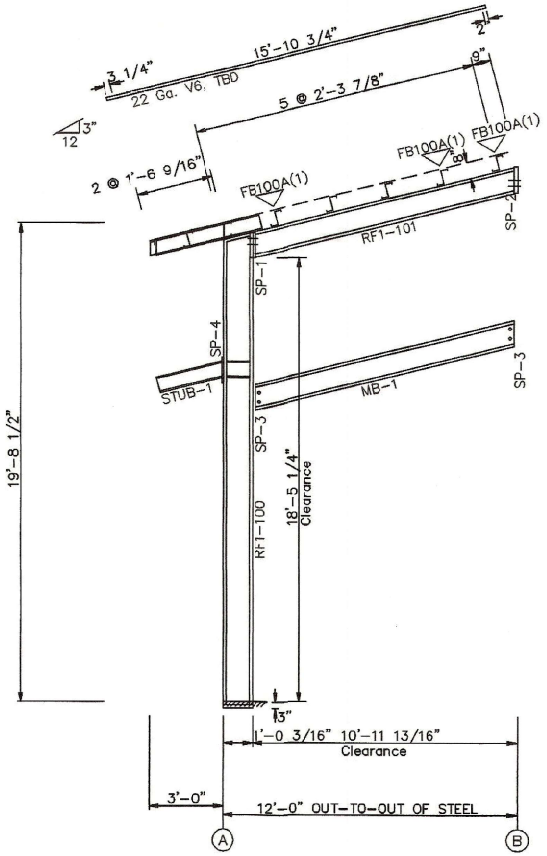
4/20/17

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	0	0	A325	0.500	1.75
SP-2	4	0	0	A325	0.500	1.25
SP-3	0	2	0	A325	0.750	1.75
SP-4	2	4	2	A325	0.500	1.50

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A – L15X1/8

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-100	W12X26	19'-3 1/4"
RF1-101	W10X12	11'-3 13/16"
STUB-1	W8X10	4'-3"
MB-1	W12X16	12'-4"



BUILDING CROSS SECTION: FRAME LINE 5.1

GENERAL NOTES:

1. See Detail Sheets for Connection Information.
2. See Shipping List for Flange Brace Lengths.

CORE BUILDING SYSTEMS
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LAKE TECHNICAL COLLEGE BLDG 2
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DATE: 3/ 7/17 | REVISION: 01
ENG: TBT | DWN: BJC | APPD: TBT

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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

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1	SEE CO-01	4/20/17

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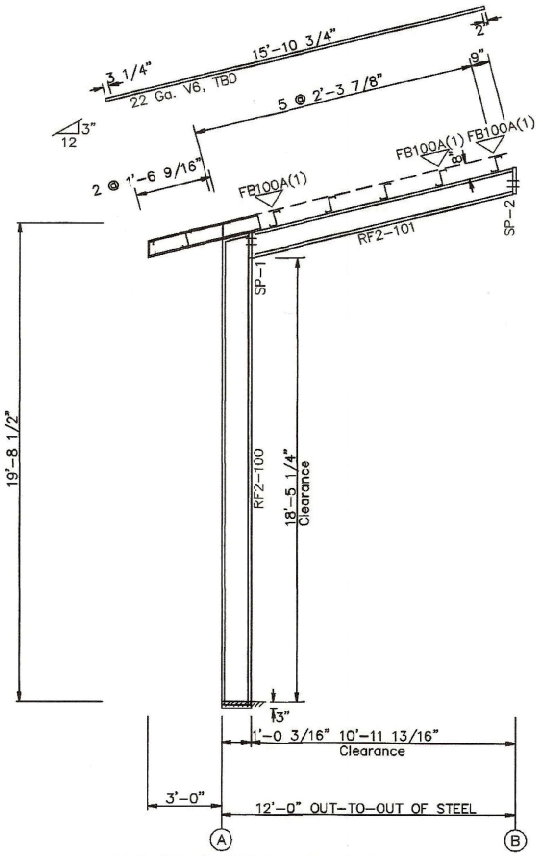
4/20/17

SPLICE BOLT TABLE						
Mark	Cty Top	Bot	Int	Type	Dia	Length
SP-1	4	0	0	A325	0.500	1.75
SP-2	4	0	0	A325	0.500	1.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A – L15X1/8

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF2-100	W12X26	19'-3 1/4"
RF2-101	W10X12	11'-3 13/16"



BUILDING CROSS SECTION: FRAME LINE 4

GENERAL NOTES:

1. See Detail Sheets for Connection Information.
2. See Shipping List for Flange Brace Lengths.

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LAKE TECHNICAL COLLEGE BLDG 2
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DATE: 3/7/17 | REVISION: 01
ENG: TBT | DWN: BJC | APPD: TBT

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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
1	SEE CO 01	4/20/17

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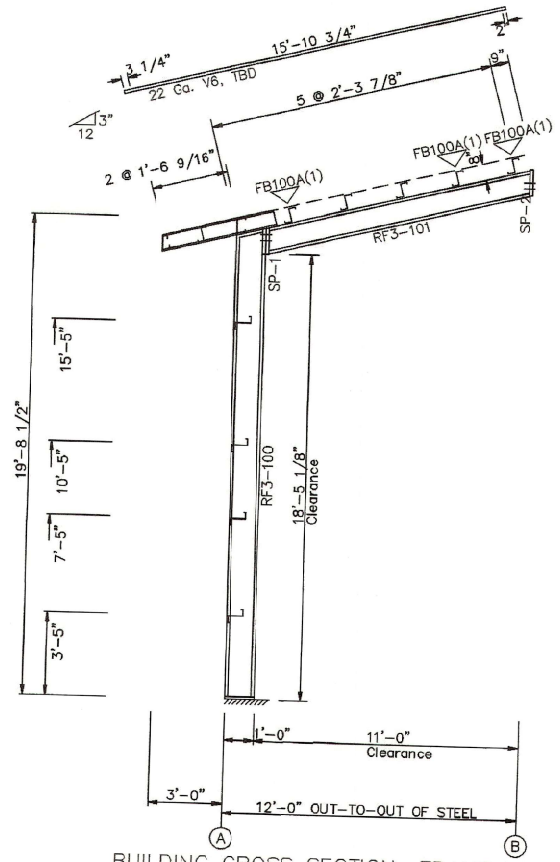
STATE OF FLORIDA
PROFESSIONAL ENGINEER
4/20/17

SPLICE BOLT TABLE						
Mark	Cty Top	Bct	Int	Type	Dia	Length
SP-1	4	C	0	A325	0.500	1.50
SP-2	4	C	0	A325	0.500	1.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A – L15X1/8

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF3-100	W12X16	19'-0 1/4"
RF3-101	W10X12	11'-4 1/16"



BUILDING CROSS SECTION: FRAME LINE 3

GENERAL NOTES:

1. See Detail Sheets for Connection Information.
2. See Shipping List for Flange Brace Lengths.

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LAKE TECHNICAL COLLEGE BLDG 2
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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
1	SEE CO-01	4/20/17

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LICENSE
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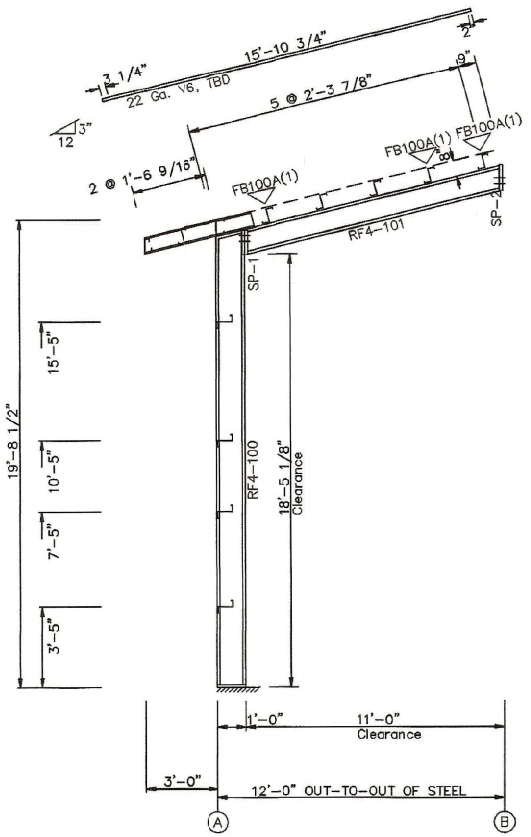
4/20/17

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	0	0	A325	0.500	1.50
SP-2	4	0	0	A325	0.500	1.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A – L15X1/8

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF4-100	W12X16	19'-0 1/4"
RF4-101	W10X12	11'-4 1/16"



BUILDING CROSS SECTION: FRAME LINE 2

GENERAL NOTES:

1. See Detail Sheets for Connection Information.
2. See Shipping List for Flange Brace Lengths.

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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

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STATE OF FLORIDA
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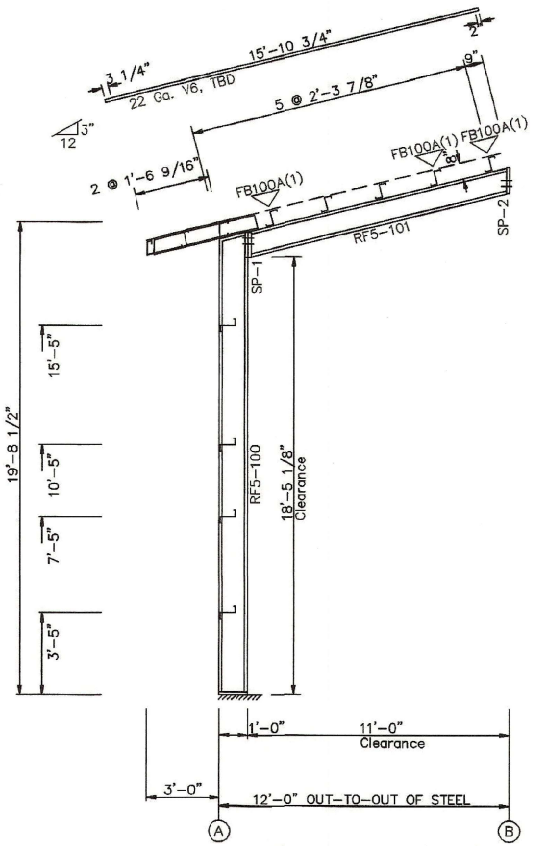
4/20/17

SPLICE BOLT TABLE						
Mark	Qty Top	Bot	Int	Type	Dia	Length
SP-1	4	0	0	A325	0.500	1.50
SP-2	4	0	0	A325	0.500	1.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A – L15X1/8

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF5-100	W12X16	19'-0 1/4"
RF5-101	W10X12	11'-4 1/16"



BUILDING CROSS SECTION: FRAME LINE 1

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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

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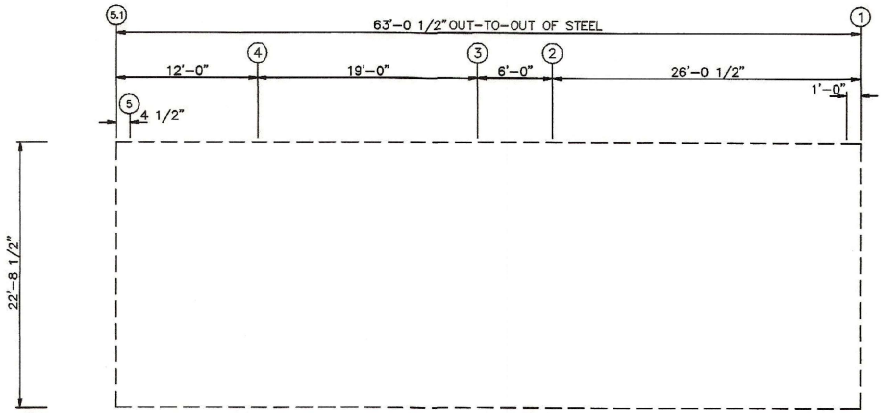
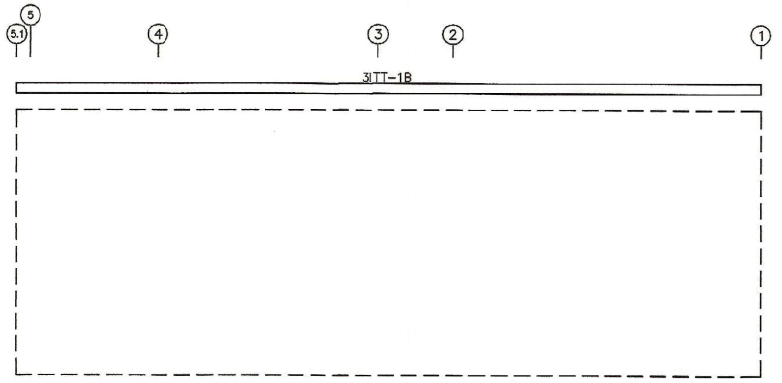


SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

TRIM COLORS			
EAVE TRIM	= TBD	CORNER TRIM	= TBD
BASE TRIM	= TBD	GUTTER	= TBD
DOOR TRIM	= TBD	DOWNSPOUTS	= TBD
RAKE TRIM	= TBD		
* LINER TRIM	= Liner panel color		
* SOFFIT TRIM	= Soffit panel color		

* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.

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LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
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DRAWING STATUS

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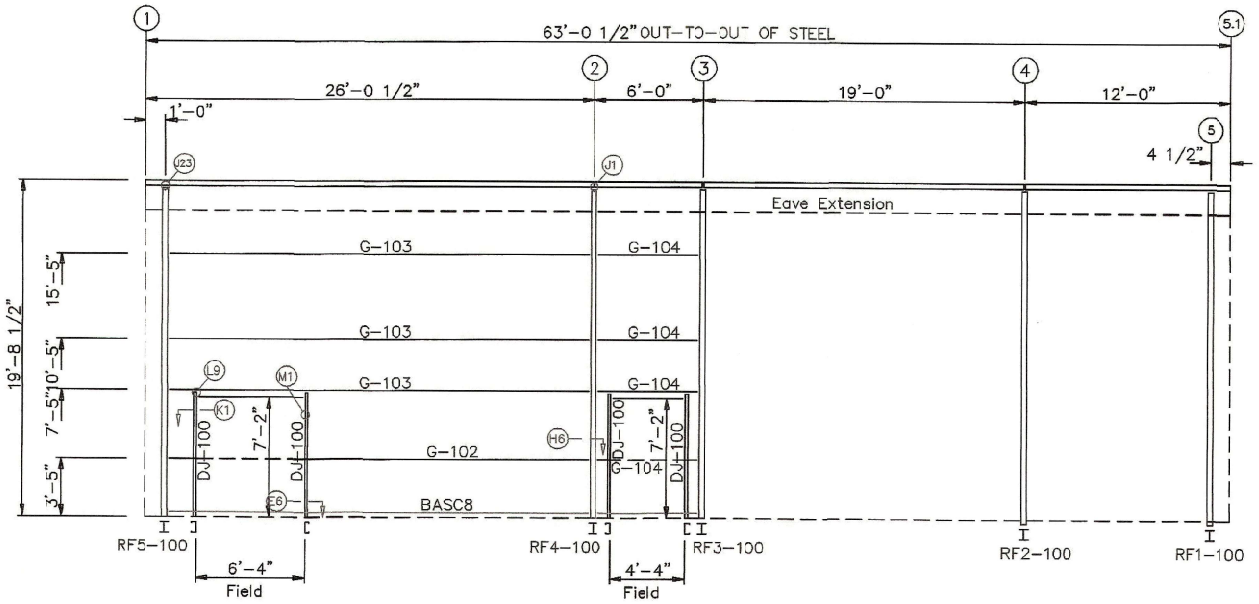
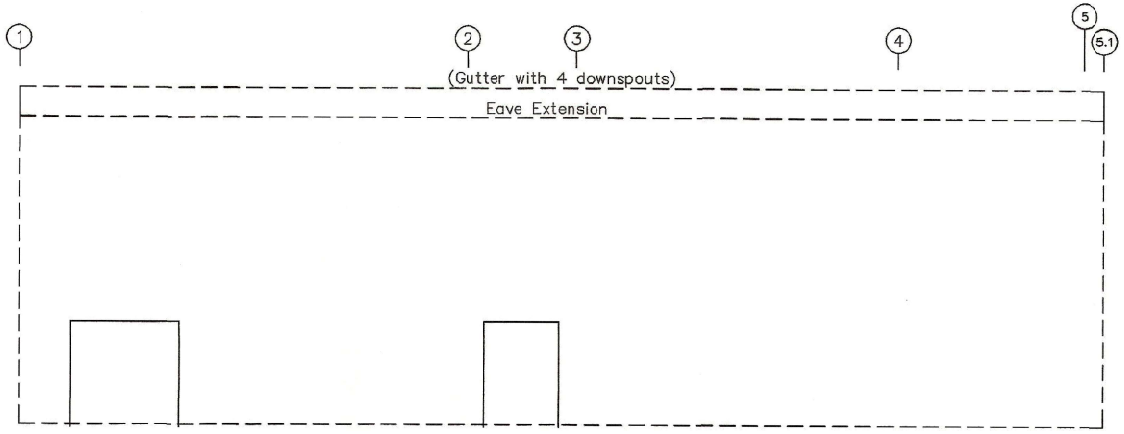


SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: MBCI 7.2 Ga. 24 – TBD

MEMBER TABLE
FRAME LINE A

QUAN	MARK	PART	LENGTH
4	DJ-100	8X35C16	7'-4 3/4"
1	G-102	8X25Z14	24'-6"
3	G-103	8X25Z10	24'-6"
4	G-104	8X25Z16	5'-11 1/2"

GENERAL NOTES:
1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

TRIM COLORS

EAVE TRIM	= TBD	CORNER TRIM	= TBD
BASE TRIM	= TBD	GUTTER	= TBD
DOOR TRIM	= TBD	DOWNSPOUTS	= TBD
RAKE TRIM	= TBD		
* LINER TRIM	= Liner panel color		
* SOFFIT TRIM	= Soffit panel color		

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LAKE TECHNICAL COLLEGE BLDG 2
12'-0" x 63'-0 1/2" x 19'-8 1/2" x 22'-8
DATE: 3/ 7/17 | REVISION: 01
ENG: TBT | DWN: BJC | APPD: TBT

F.O. 20468

LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
1	SEE CO-01	4/20/17

DRAWING STATUS

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LICENSE
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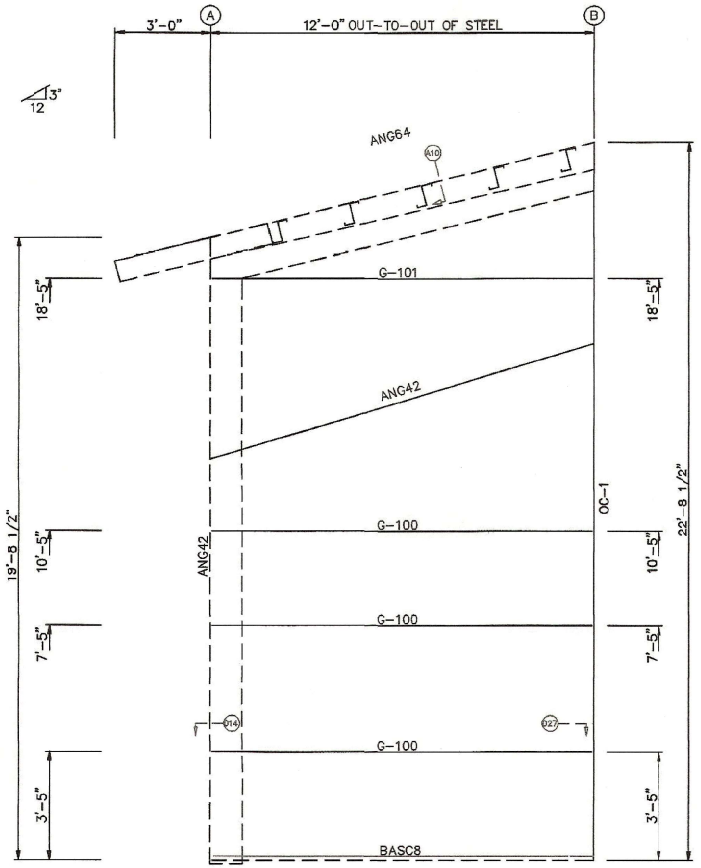
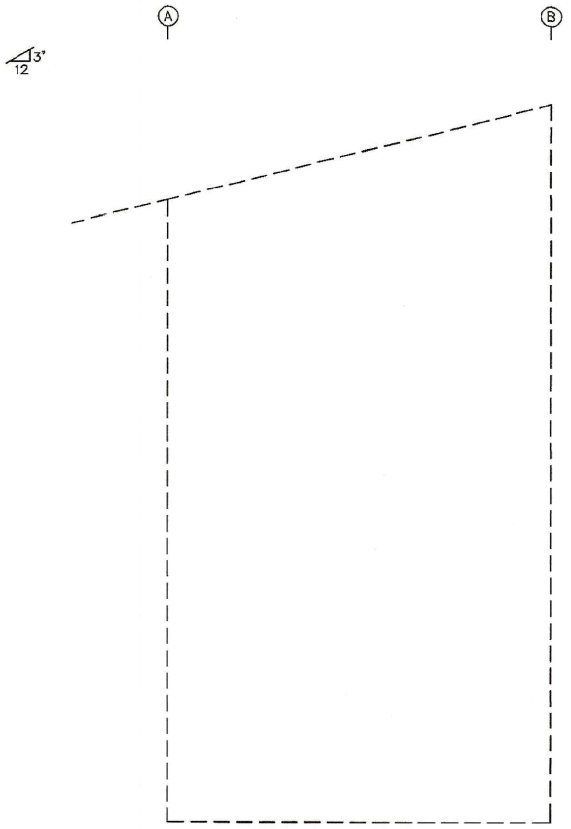
PROFESSIONAL ENGINEER
4/20/17

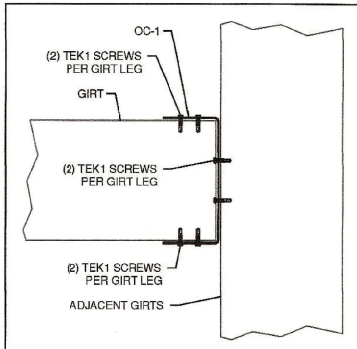
MEMBER TABLE FRAME LINE 5.1			
QUAN	MARK	PART	LENGTH
1	OC-1	8X30C16	22'-8"
3	G-100	8X25Z16	11'-11 1/2"
1	G-101	8X25Z14	11'-11 1/2"



ENDWALL FRAMING: FRAME LINE 5.1



ENDWALL SHEETING & TRIM: FRAME LINE 5.1
PANELS: MBCI 7.2 Ga. 24 – TBD



GIRT CONNECTION (TYP.)

TRIM COLORS			
EAVE TRIM	= TBD	CORNER TRIM	= TBD
BASE TRIM	= TBD	GUTTER	= TBD
DOOR TRIM	= TBD	DOWNSPOUTS	= TBD
RAKE TRIM	= TBD		
* LINER TRIM	= Liner panel color		
* SOFFIT TRIM	= Soffit panel color		

* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. See detail C7A for field coping of coldform endwall column flange braces.
3. All connections to door or window jambs where the clip is not desigrated in the clip table / drawing are made with JC# clips (#= Girt Depth).

CORE BUILDING SYSTEMS
404 Sarah Furnace Road - Imler, PA 16655 (814) 276 - 9811

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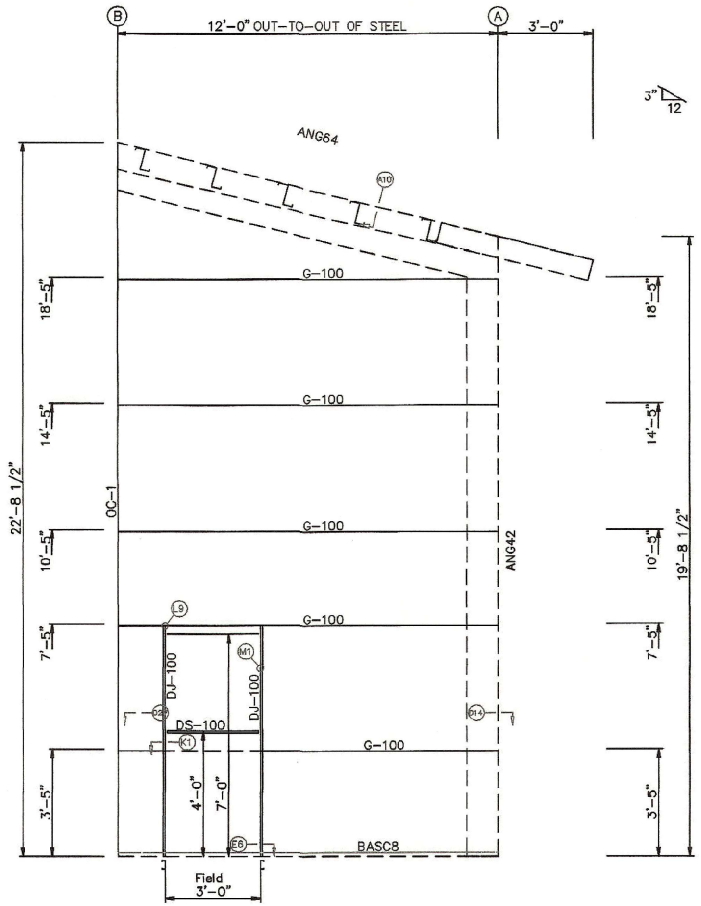
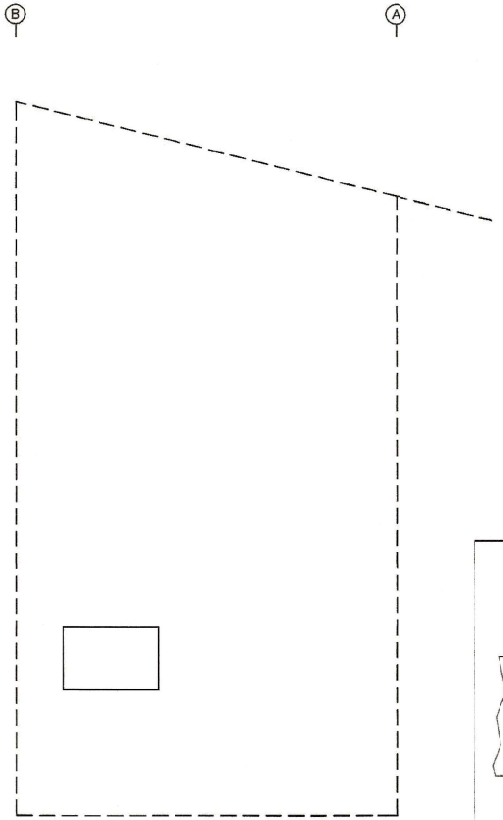
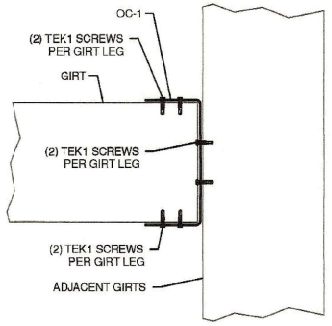
MEMBER TABLE FRAME LINE 1			
QUAN	MARK	PART	LENGTH
1	OC-1	8X30C16	22'-8"
2	DJ-100	8X35C16	7'-4 3/4"
1	DS-100	8X35C16	3'-0"
5	G-100	8X25Z16	11'-11 1/2"



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: MBCI 7.2 Ga. 24 – TBD



GIRT CONNECTION (TYP.)

TRIM COLORS			
EAVE TRIM	= TBD	CORNER TRIM	= TBD
BASE TRIM	= TBD	GUTTER	= TBD
DOOR TRIM	= TBD	DOWNSPOUTS	= TBD
RAKE TRIM	= TBD		
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3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

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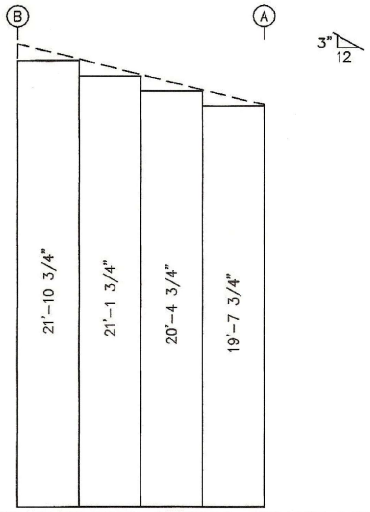
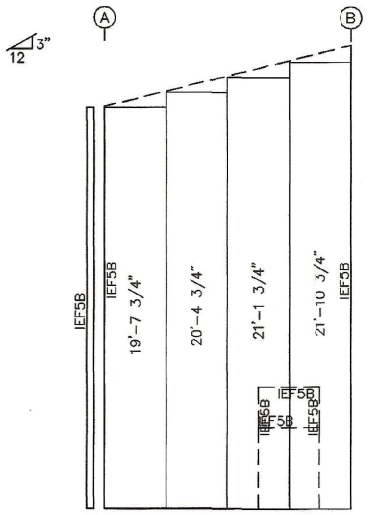
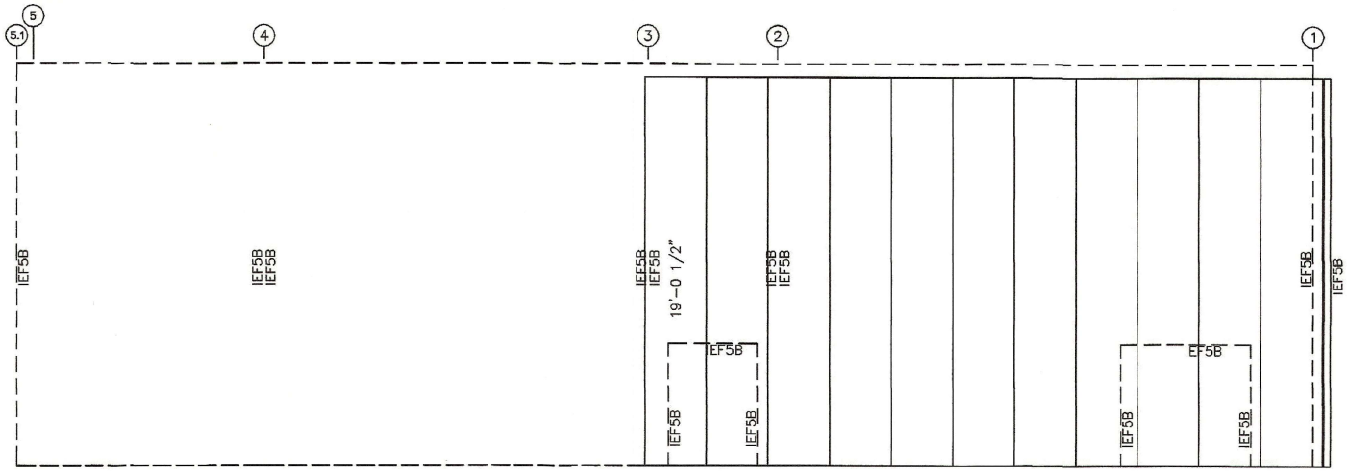


ENDWALL LINER SHEETING & TRIM: FRAME LINE 5.
PANELS: 24 Ga. R – TBD
(As Viewed From Inside Of Building)



ENDWALL LINER SHEETING & TRIM: FRAME LINE 1
PANELS: 24 Ga. R – TBD
(As Viewed From Inside Of Building)



SIDEWALL LINER SHEETING & TRIM: FRAME LINE A
PANELS: 24 Ga. R – TBD
(As Viewed From Inside Of Building)

TRIM COLORS	
EAVE TRIM = TBD	CORNER TRIM = TBD
BASE TRIM = TBD	GUTTER = TBD
DOOR TRIM = TBD	DOWNSPOUTS = TBD
RAKE TRIM = TBD	
* LINER TRIM = Liner panel color	
* SOFFIT TRIM = Soffit panel color	

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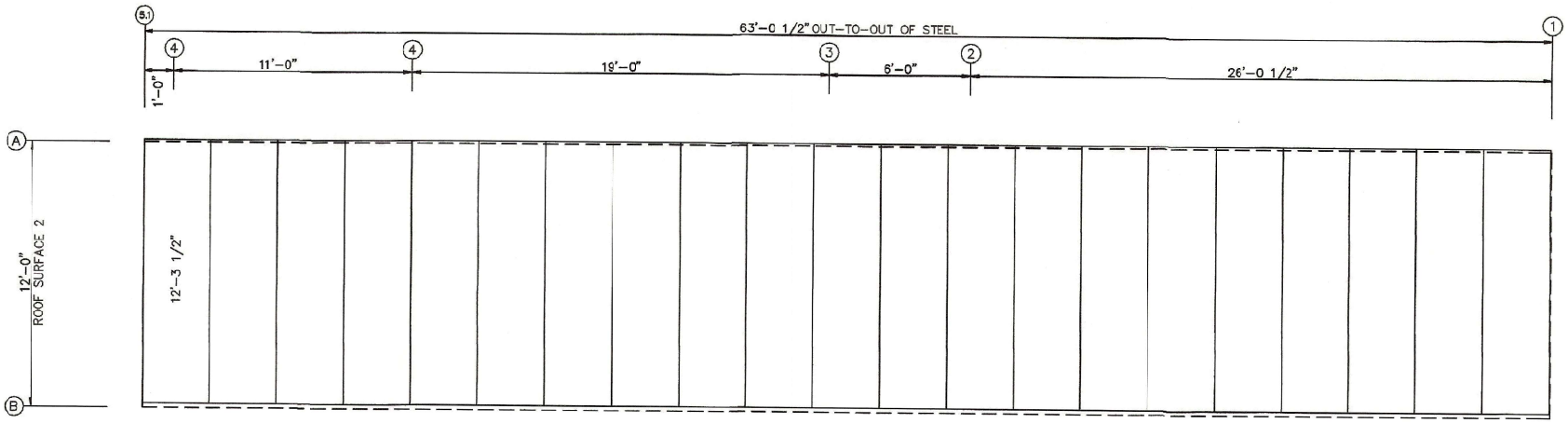

ROOF LINER SHEETING PLAN
PANELS: 24 Ga. R - TBD

TRIM COLORS			
EAVE TRIM	= TBD	CORNER TRIM	= TBD
BASE TRIM	= TBD	GUTTER	= TBD
DOOR TRIM	= TBD	DOWNSPOUTS	= TBD
RAKE TRIM	= TBD		
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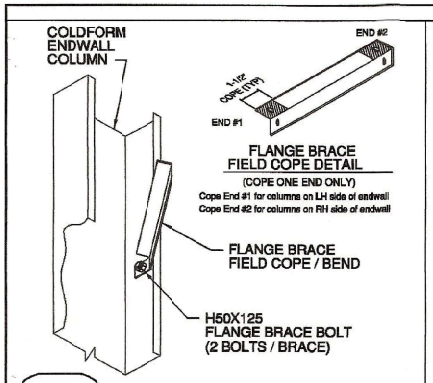
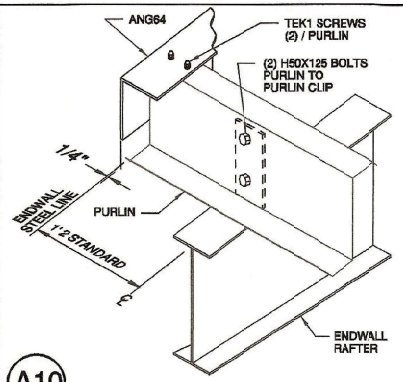
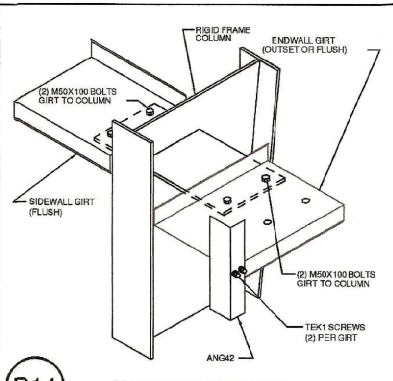


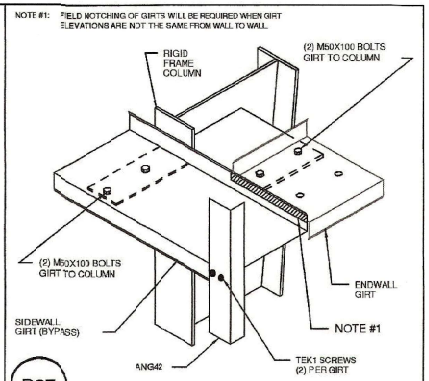
C7A FLANGE BRACE TO BYPASS COLDFORM ENDWALL COLUMN



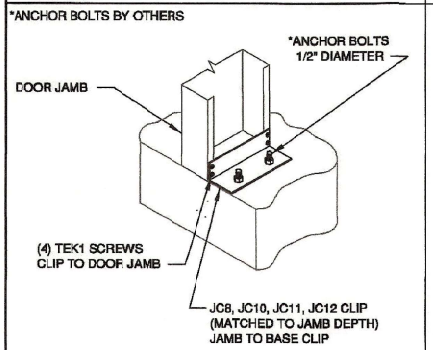
A10 PURLIN TO RIGID FRAME ENDWALL RAFTER CONNECTION



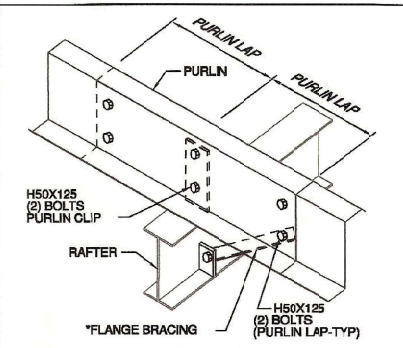
D14 RIGID FRAME IN ENDWALL COLUMN TO FLUSH SIDEWALL GIRT



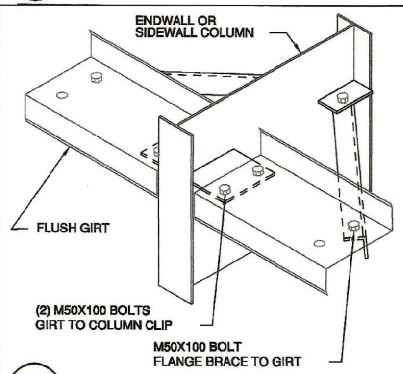
D27 RIGID FRAME IN ENDWALL TO FLUSH OR BYPASS ENDWALL GIRT



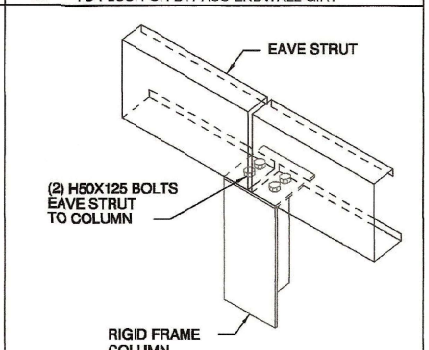
E6 DOOR JAMB TO FOUNDATION



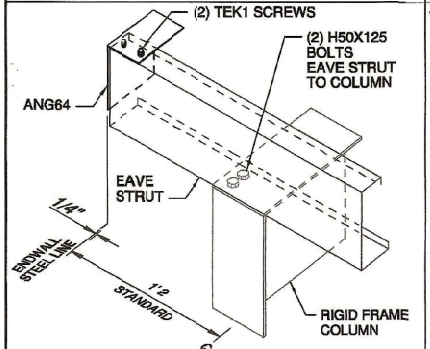
G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



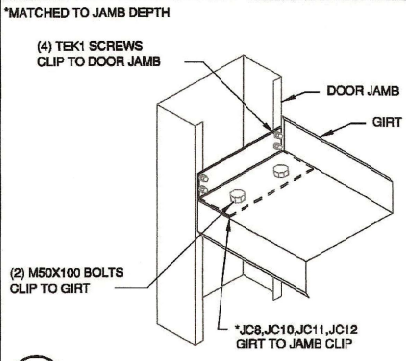
H6 GIRT TO COLUMN - FLUSH GIRTS



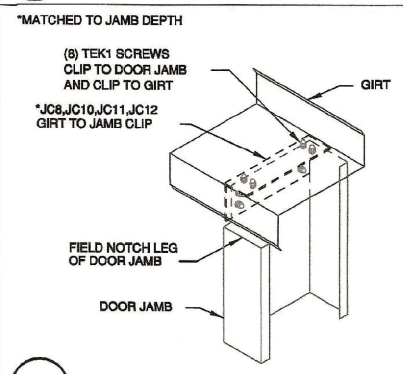
J1 EAVE STRUT TO RIGID FRAME FLUSH GIRT CONDITION



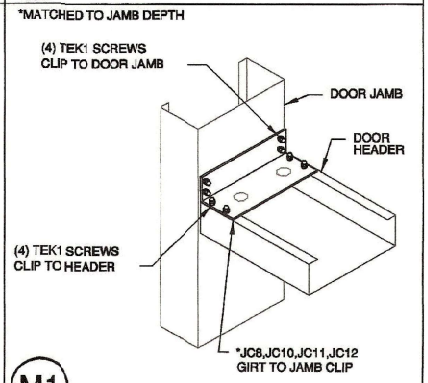
J23 EAVE STRUT TO RIGID FRAME IN ENDWALL FLUSH GIRT CONDITION



K1 WALL GIRT TO DOOR JAMB



L9 DOOR JAMB TO WALL GIRT



M1 DOOR HEADER TO DOOR JAMB

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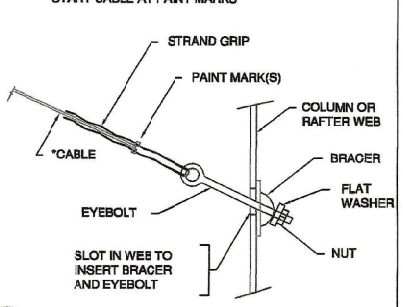
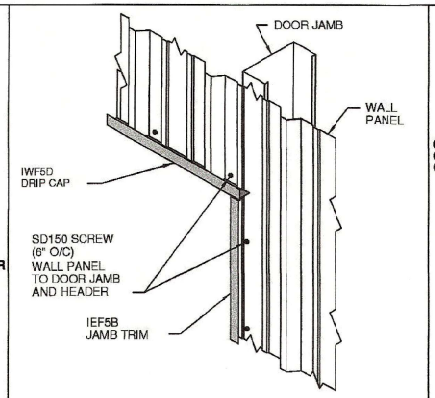
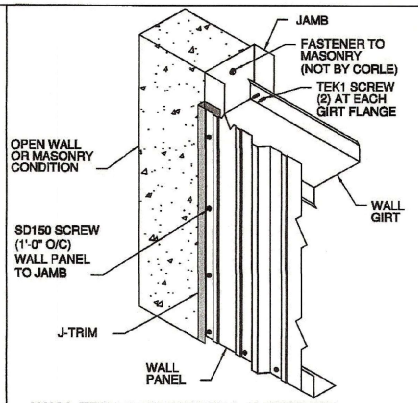


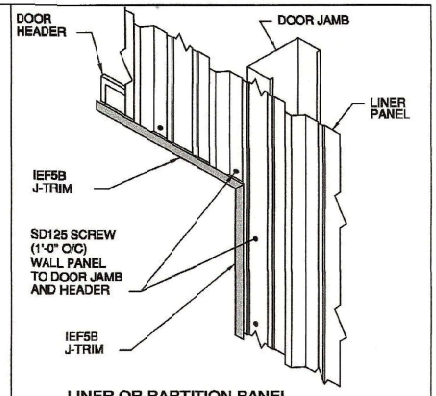
Q2 CABLE BRACE / EYEBOLT



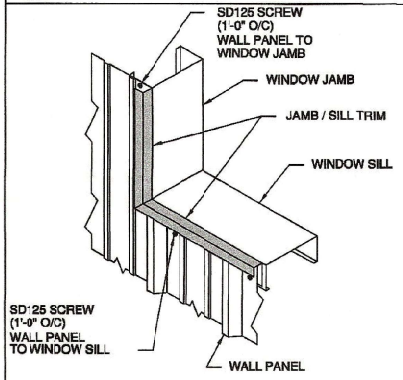
DOOR & WINDOW JAMB / HEADER TRIM



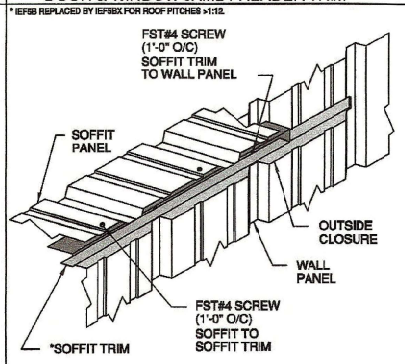
WALL TRIM AT OPEN WALL / MASONRY



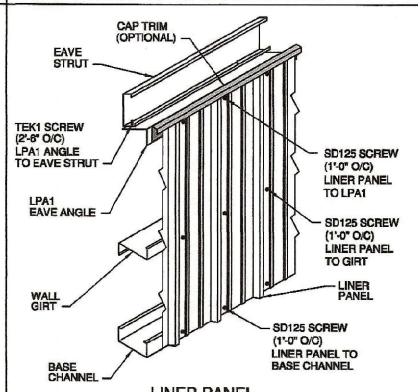
LINER OR PARTITION PANEL
DOOR & WINDOW JAMB / HEADER TRIM



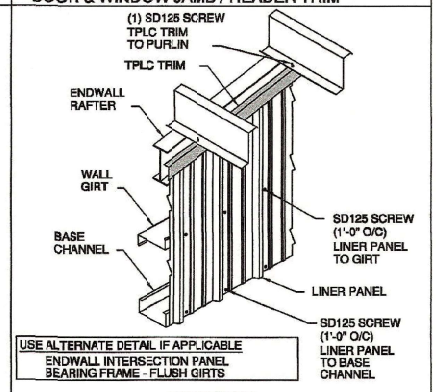
LINER OR PARTITION PANEL
WINDOW JAMB / SILL TRIM



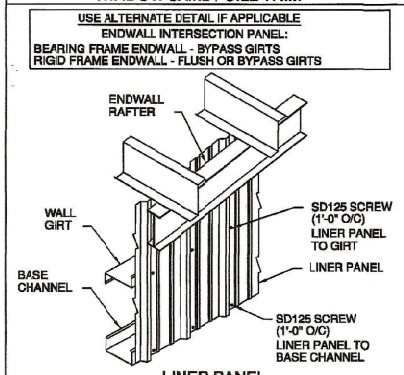
SOFFIT AT BUILDING EAVE



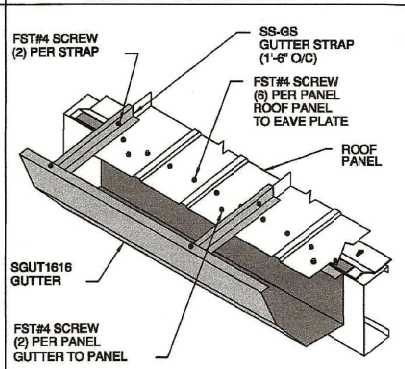
LINER PANEL
FULL HEIGHT AT SIDEWALL - LOW EAVE



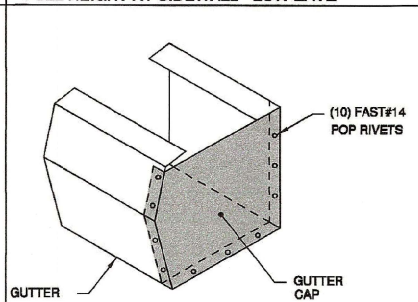
LINER PANEL
BEARING FRAME ENDWALL - FLUSH GIRTS



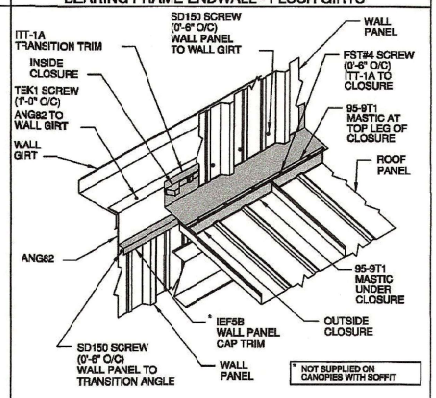
LINER PANEL
BEARING FRAME ENDWALL - BYPASS GIRTS
EXPANDABLE ENDWALL - FLUSH OR BYPASS GIRTS



GUTTER AT OPEN WALL OR ROOF EXTENSION



GUTTER CAP INSTALLATION



ROOF / WALL TRANSITION

CORLE BUILDING SYSTEMS
404 Sarah Furnace Road - Imler, PA 16655 (814) 276 - 9811

LAKE TECHNICAL COLLEGE BLDG 2
12'-0" x 63'-0 1/2" x 19'-8 1/2" x 22'-8
DATE: 3/ 7/17 | REVISION: 01
ENG: TBT | DWN: BJC | APPD: TBT

F.O. 20468

LAKE TECHNICAL COLLEGE BLDG 2

REVISION HISTORY

REV.	DESCRIPTION	DATE
1	SEE CO-01	4/20/17

DRAWING STATUS

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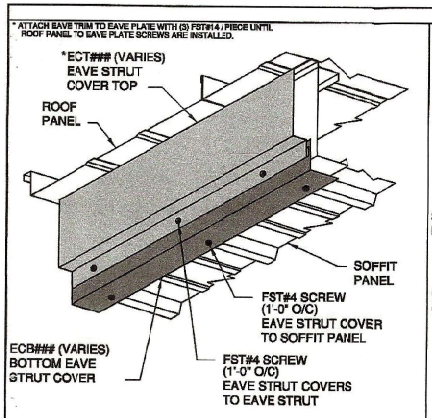
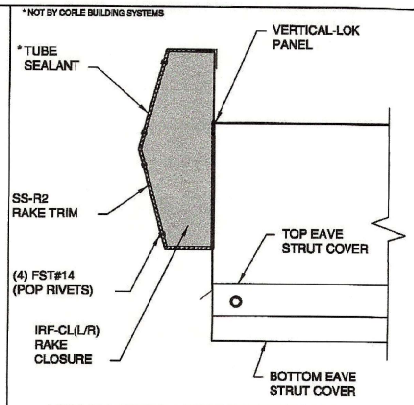
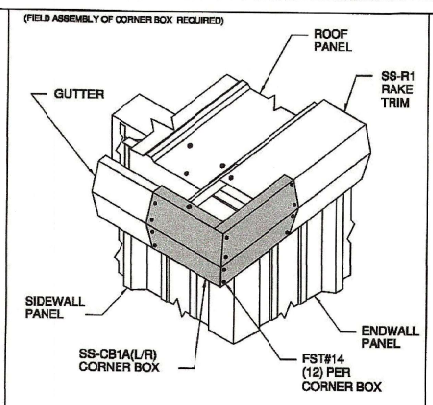


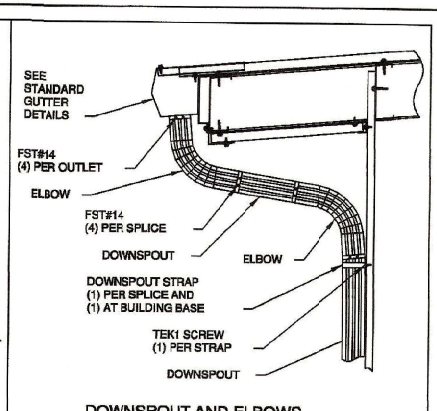
LOW EAVE EXTENSION TRIM



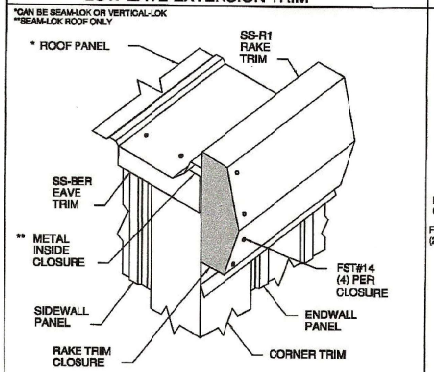
RAKE CLOSURE AT EAVE EXTENSION



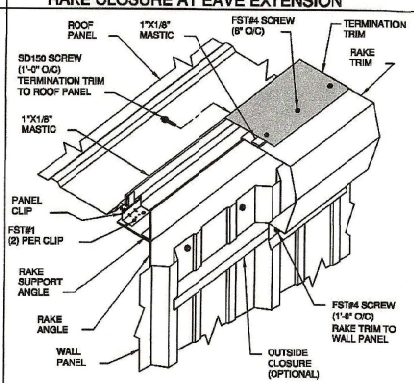
CORNER BOX INSTALLATION



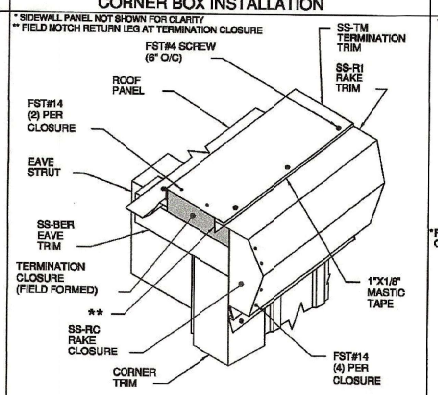
DOWNSPOUT AND ELBOWS AT EAVE ROOF EXTENSION



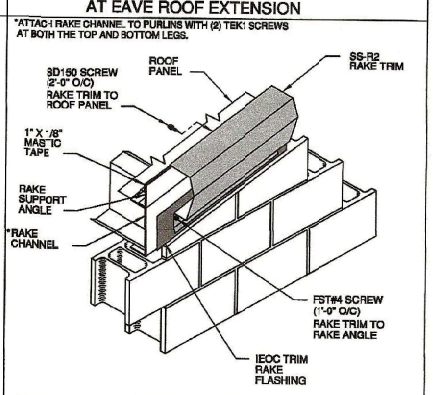
RAKE CLOSURE



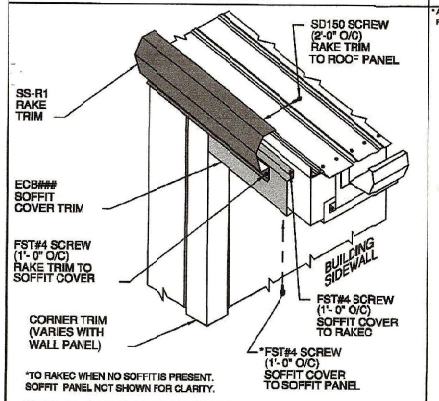
TERMINATION TRIM



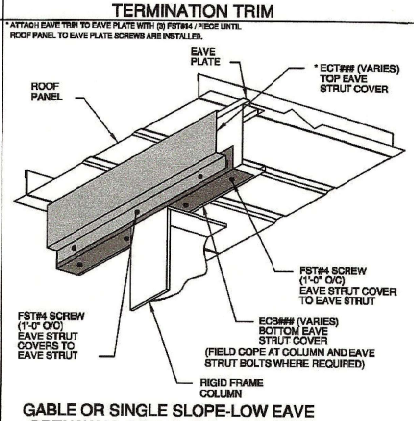
TERMINATION TRIM CLOSURE



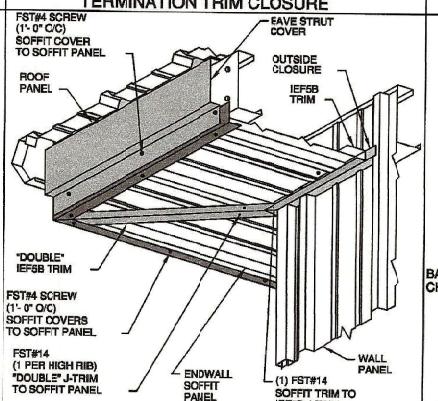
RAKE TRIM AT OPEN OR MASONRY WALL
(ROOF SURFACES </= 90' 0 EAVE TO PEAK)



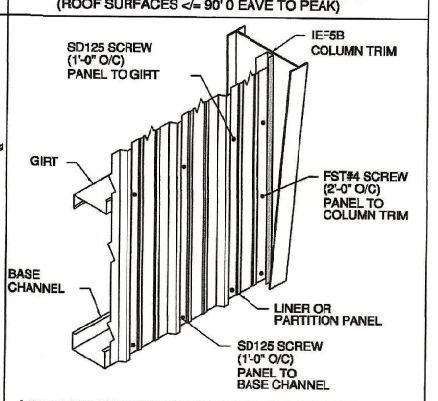
GABLE AT EAVE EXTENSION OR CANOPY



GABLE OR SINGLE SLOPE-LOW EAVE
OPEN WALL OR MASONRY CONDITION



SOFFIT TRIM AT CORNER



LINER OR PARTITION PANEL COLUMN TRIM
FLUSH GIRT CONDITION

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LAKE TECHNICAL COLLEGE BLDG 2

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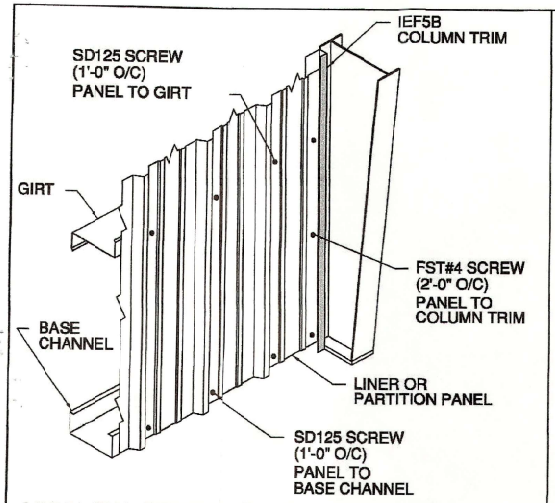
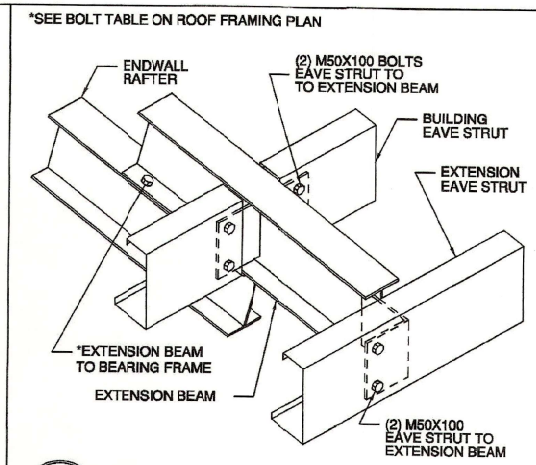
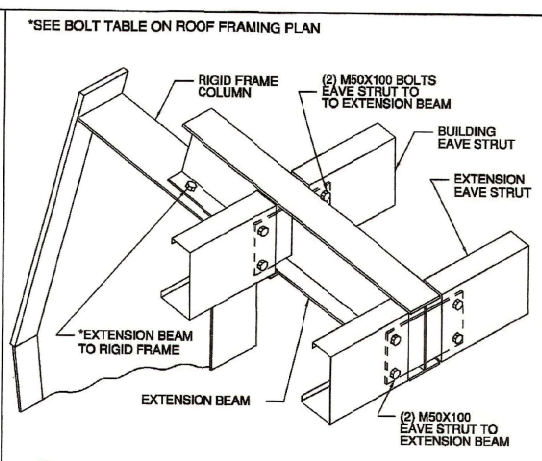


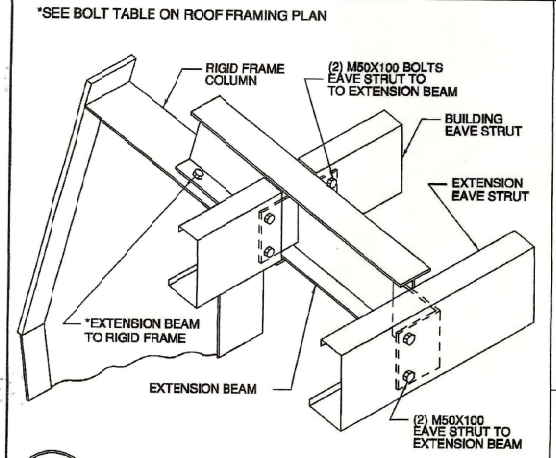
LINER OR PARTITION PANEL COLUMN TRIM
BYPASS GIRT CONDITION



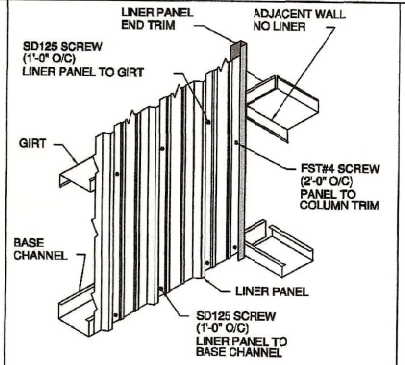
X6 FLUSH EAVE EXTENSION TO BEARING FRAME ENDWALL AT LOW EAVE



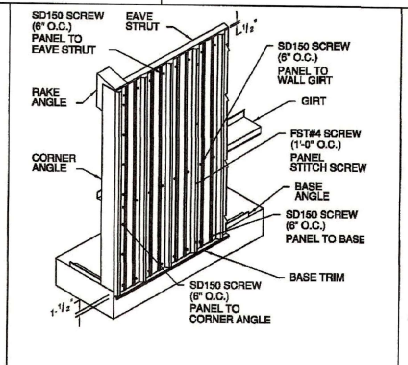
X7 FLUSH EAVE EXTENSION TO RIGID FRAME - LOW EAVE



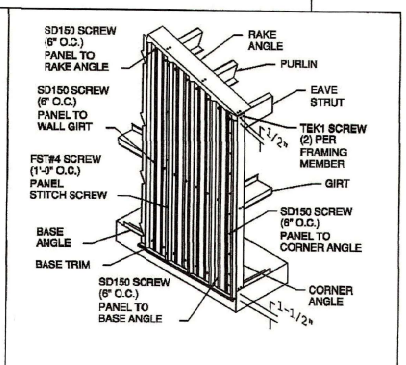
X8 FLUSH EAVE EXTENSION TO ENDWALL RIGID FRAME - LOW EAVE



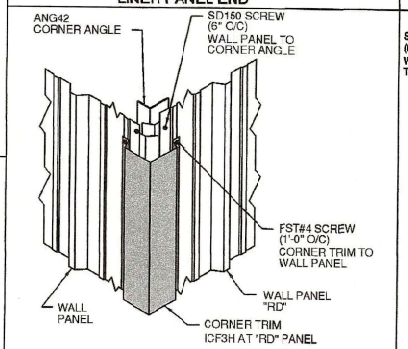
LINER PANEL END



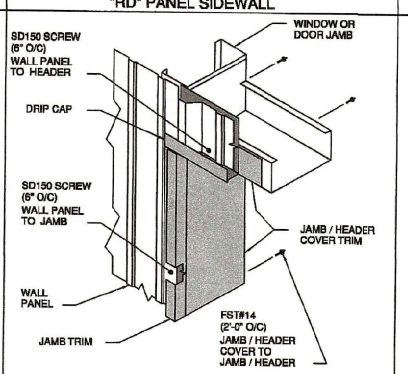
"RD" PANEL SIDEWALL



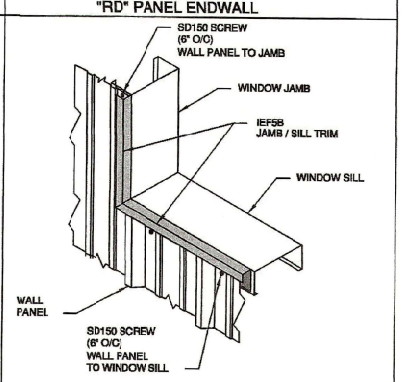
"RD" PANEL ENDWALL



CORNER TRIM



DOOR & WINDOW DELUXE JAMB TRIM



WINDOW JAMB / SILL TRIM

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LAKE TECHNICAL COLLEGE BLDG 2

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